# GROUND MOTION ON ALLUVIAL VALLEYS UNDER INCIDENT PLANE SH WAVES 

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#### Abstract

The scattering and diffraction of harmonic $S H$ waves by an arbitrarily shaped alluvial valley in a layered material is considered. The problem is solved in terms of boundary integral equations which yield a numerical solution.


## 1. Introduction

After a substantial earthquake it is often found that damage is concentrated in particular areas [5]. This may be accounted for by various causes such as poor quality of construction, local topography and the local geology. One explanation which has been put forward is that the damage distribution is caused by seismic wave amplification associated with the local topography and soil characteristics [10].

Motivated by the need to provide reliable design parameters for structures, the problem has been studied by numerous authors. In certain circumstances ground-motion amplifications can be studied adequately by simple shearbeam amplification models. However, for irregular topographies, the problem must be studied as a spatial phenomenon. The simplest models which yield significant information in this area are two-dimensional, and several studies ([1], [2], [3], [7], [8], [10]) of this type have provided a basic understanding of the problem.

Integral equation formulations have been found to be particularly useful in obtaining numerical solutions to problems of this type. In particular, Wong and Jennings [9] used singular integral equations to solve the problem of

[^0]scattering and diffraction of SH-waves by canyons of arbitrary cross section. Subsequently Wong et al. [11] extended the method of [9] to ihe case of a layer overlying a semi-infinite soil medium, and applied their analysis to modeling the variations in measured displacements during a full-scale lowamplitude wave propagation test. Also Sanchez-Sesma and Esquivel [6] considered ground motion on alluvial valleys under incident planar SH-waves.

The present work can be considered as an extension of previous work on integral equation formulations to include the case of anisotropic materials. In many cases the medium through which the waves are propagating is not homogeneous and isotropic, and is more accurately modeled as an anisotropic material. In particular, the current work examines the effect of anisotropy on ground motion on alluvial valleys under incident planar SH waves. Numerical results are obtained, and these are compared with those given in [6] for isotropic materials.

## 2. Statement of the problem

Referring to a Cartesian frame $O x_{1} x_{2} x_{3}$, consider an anisotropic elastic half-space occupying the region $x_{2}>0$. The half-space is divided into two regions which contain different homogeneous anisotropic materials (see Figure 1). The materials are assumed to adhere rigidly to each other so that the displacement and stress are continuous across the interface. Also, the geometries of the two regions are assumed not to vary in the $O x_{2}$ direction, and the boundary $x_{2}=0$ is traction free.

A horizontally polarised SH wave propagates towards the surface of the elastic half-space. This is in the form of a plane wave with unit amplitude,


Figure 1. The alluvial valley and surrounding half-space.
and gives rise to a displacement field

$$
\begin{equation*}
u_{3 I}^{(1)}=\exp i \omega\left(t+\frac{x_{1}}{c_{1}}+\frac{x_{2}}{c_{2}}\right) \tag{1}
\end{equation*}
$$

where $\omega$ is the circular frequency, $c_{1}$ and $c_{2}$ are constants and $u_{3}^{(1)}$ denotes the displacement in the $O x_{3}$ direction in region 1 (see Figure 1). The problem is to determine the displacements associated with the reflected, diffracted and refracted waves.

## 3. Integral equation formulation

Since the incident wave is of the form (1), and the geometry does not vary in the $O x_{3}$ direction, a solution of the problem can be obtained in terms of plane polarised SH waves. For such waves, the only nonzero displacement in this case is $u_{3}$, which must satisfy the equation of motion for antiplane elastic deformations of anisotropic materials. That is,

$$
\begin{equation*}
\lambda_{i j}^{(\alpha)} \frac{\partial^{2} u_{3}^{(\alpha)}}{\partial x_{i} \partial x_{j}}=\rho^{(\alpha)} \frac{\partial^{2} u_{3}^{(\alpha)}}{\partial t^{2}} \quad \text { for } \alpha=1,2 \tag{2}
\end{equation*}
$$

where $u_{3}^{(1)}$ and $u_{3}^{(2)}$ denote the displacements in regions 1 and 2 respectively. Also $\lambda_{i j}^{(\alpha)}$ denote the elastic moduli, which must satisfy the symmetry conditions $\lambda_{i j}^{(\alpha)}=\lambda_{j i}^{(\alpha)}, \rho^{(\alpha)}$ denotes the density, $t$ denotes the time, and summation from 1 to 2 is assumed for repeated Latin indices only.

In view of the form of the incident plane wave (1), a solution to (2) is sought for which the displacement has a time dependence of the form $\exp (i \omega t)$ so that

$$
\begin{equation*}
u_{3}^{(\alpha)}\left(x_{1}, x_{2}, t\right)=u^{(\alpha)}\left(x_{1}, x_{2}\right) \exp (\mathrm{i} \omega t) \tag{8}
\end{equation*}
$$

Equation (3) provides a solution to (2) if $u^{(\alpha)}$ satisfies the equation

$$
\begin{equation*}
\lambda_{i j}^{(\alpha)} \frac{\partial^{2} u^{(\alpha)}}{\partial x_{i} \partial x_{j}}+\rho^{(\alpha)} \omega^{2} u^{(\alpha)}=0 \tag{4}
\end{equation*}
$$

Suppose the incident wave (1) has an angle of incidence $\gamma_{I}$ (Figure 2). Then $c_{1}=\beta^{(1)} / \sin \gamma_{I}$ and $c_{2}=\beta^{(1)} / \cos \gamma_{I}$ where $\beta^{(1)}$ is a constant. Now $u_{3}^{(1)}$ as given by (1) must satisfy (2) so that

$$
\begin{equation*}
\left[\beta^{(1)}\right]^{2}=\frac{\lambda_{11}^{(1)} \sin ^{2} \gamma_{I}+2 \lambda_{12}^{(1)} \sin \gamma_{I} \cos \gamma_{I}+\lambda_{22}^{(1)} \cos ^{2} \gamma_{I}}{\rho^{(1)}} \tag{5}
\end{equation*}
$$

where $\beta^{(1)}$ is the wave velocity of the incident wave.


Figure 2. Angles of incidence and reflection.
Consider the case when regions 1 and 2 are occupied by the same material. In order to satisfy the traction-free surface condition on $x_{2}=0$, it is necessary to have a reflected wave of the form

$$
\begin{equation*}
u_{3 R}^{(1)}=\operatorname{expi} \omega\left(t+\frac{x_{1}}{c_{1}^{\prime}}-\frac{x_{2}}{c_{2}^{\prime}}\right) . \tag{6}
\end{equation*}
$$

The displacement $u_{3 R}^{(1)}$ in the half-space is given by the sum of the displacements given by (1) and (6). Thus

$$
\begin{equation*}
u_{3}^{(1)}=u_{3 R}^{(1)}=\exp i \omega\left(t+\frac{x_{1}}{c_{1}}+\frac{x_{2}}{c_{2}}\right)+\exp \mathrm{i} \omega\left(t+\frac{x_{1}}{c_{1}^{\prime}}-\frac{x_{2}}{c_{2}^{\prime}}\right) . \tag{7}
\end{equation*}
$$

The stresses are given by

$$
\begin{equation*}
\sigma_{i 3}^{(\alpha)}=\lambda_{i j}^{(\alpha)} \frac{\partial u_{3}^{(\alpha)}}{\partial x_{j}} \tag{8}
\end{equation*}
$$

so that the stress $\sigma_{23}^{(1)}$ on $x_{2}=0$ is

$$
\begin{equation*}
\sigma_{23}^{(1)}=\left(\frac{\lambda_{21}^{(1)}}{c_{1}^{\prime}}-\frac{\lambda_{22}^{(1)}}{c_{2}^{\prime}}\right) \exp \left[\mathrm{i} \omega\left(t+\frac{x_{1}}{c_{1}^{\prime}}\right)\right]+\left(\frac{\lambda_{21}^{(1)}}{c_{1}}+\frac{\lambda_{22}^{(1)}}{c_{2}}\right) \exp \left[\mathrm{i} \omega\left(t+\frac{x_{1}}{c_{1}}\right)\right] . \tag{9}
\end{equation*}
$$

This stress will be zero for all time $t$ if

$$
c_{1}^{\prime}=c_{1}
$$

and

$$
-\frac{\lambda_{21}^{(1)}}{c_{1}}+\frac{\lambda_{22}^{(1)}}{c_{2}^{\prime}}=\frac{\lambda_{21}^{(1)}}{c_{1}}+\frac{\lambda_{22}^{(1)}}{c_{2}}
$$

or

$$
\begin{equation*}
\frac{1}{c_{2}^{\prime}}=\frac{1}{c_{2}}+\frac{2 \lambda_{21}^{(1)}}{\lambda_{22}^{(1)} c_{1}} \tag{10}
\end{equation*}
$$

This equation serves to provide $c_{2}^{\prime}$ in terms of the unknown quantities $c_{2}, c_{1}, \lambda_{21}^{(1)}$ and $\lambda_{22}^{(1)}$. Note that if (6) is substituted into (2) then since it represents a solution to (2) it follows that

$$
\begin{equation*}
\frac{\lambda_{11}^{(1)}}{c_{1}^{2}}-\frac{2 \lambda_{12}^{(1)}}{c_{1} c_{2}^{\prime}}+\frac{\lambda_{22}^{(1)}}{c_{2}^{\prime 2}}=\rho^{(1)} \tag{11}
\end{equation*}
$$

and if $(10)$ is used to substitute for $1 / c_{2}^{\prime}$ in (11), and then into (5), so that (10) ensures (6) is a solution to (2) on the assumption that (1) is also a solution to (2).

Let $c_{1}=\beta^{\prime} / \sin \left(\gamma_{R}\right)$ and $c_{2}^{\prime}=\beta^{\prime} / \cos \left(\gamma_{R}\right)$ where $\gamma_{R}$ is the angle of reflection (see Figure 2). Then

$$
\begin{equation*}
\tan \left(\gamma_{R}\right)=\frac{c_{2}^{\prime}}{c_{1}}=\frac{\tan \left(\gamma_{I}\right)}{1+2\left(\lambda_{12}^{(1)} / \lambda_{22}^{(1)}\right) \tan \left(\gamma_{I}\right)} \tag{12}
\end{equation*}
$$

and once $\gamma_{R}$ has been determined from this equation, the wave speed $\beta^{\prime}$ of the reflected wave may be readily determined from the equation $\beta=$ $c_{1} \sin \left(\gamma_{R}\right)$.

To include the influence of a different anisotropic material in region 2 let the displacement in region 1 be given by (3) with

$$
\begin{equation*}
u^{(1)}=u_{0}^{(1)}+u_{d}^{(1)} \tag{13}
\end{equation*}
$$

where $u_{0}^{(1)}$ denotes the displacement obtained from (7) while $u_{d}^{(1)}$ denotes the displacement due to diffracted waves. The displacement in region 2 is given by (3) with $u^{(2)}=u_{r}^{(2)}$ denoting the displacement associated with the refracted waves.

In order to find $u_{d}^{(1)}$ and $u_{r}^{(2)}$, it is convenient to obtain an integral equation solution of (4). To derive the integral equation, first consider the inhomogeneous equation associated with (4). That is,

$$
\begin{equation*}
\lambda_{i j}^{(\alpha)} \frac{\partial^{2} u^{(\alpha)}}{\partial x_{i} \partial x_{j}}+\rho^{(\alpha)} \omega^{2} u^{(\alpha)}=h^{(\alpha)}\left(x_{1}, x_{2}\right) \tag{14}
\end{equation*}
$$

where $h^{(\alpha)}\left(x_{1}, x_{2}\right)$ is a given function. Any two solutions $U^{(\alpha)}$ and $V^{(\alpha)}$ of (14) are related by the integral equation
$\int_{\partial \Omega}\left(\lambda_{i j}^{(\alpha)} \frac{\partial U^{(\alpha)}}{\partial x_{j}} n_{i} V \lambda^{(\alpha)}-\lambda_{i j}^{(\alpha)} \frac{\partial V^{(\alpha)}}{\partial x_{j}} n_{i} U^{(\alpha)}\right) d s=\int_{\Omega}\left(h_{U}^{(\alpha)} V^{(\alpha)}-h_{V}^{(\alpha)} U^{(\alpha)}\right) d v$
where $\Omega$ is the region under consideration and $\Omega$ has a boundary $\partial \Omega$ with an outward pointing normal $\mathrm{n}=\left(n_{1}, n_{2}\right)$. Also $h_{U}^{(\alpha)}$ and $h_{V}^{(\alpha)}$ denote the
right-hand side of (14) corresponding to the solutions $U^{(a)}$ and $V^{(\alpha)}$ respectively.

Now suppose $h_{V}^{(\alpha)}=\delta\left(\mathbf{x}-\mathbf{x}_{0}\right)$ where $\delta$ denote the Dirac delta function, $\mathbf{x}=\left(x_{1}, x_{2}\right)$ and $\mathbf{x}_{0}=(a, b)$ where $\mathbf{x} \in \Omega$ and $\mathbf{x}_{0} \in \Omega$. Then if $u^{(\alpha)}(a, b)=U^{(\alpha)},(15)$ provides

$$
\begin{equation*}
C u^{(\alpha)}(a, b)=\int_{\partial \Omega}\left[\lambda_{i j}^{(\alpha)} \frac{\partial V^{(\alpha)}}{\partial x_{j}} n_{i} u^{(\alpha)}-\lambda_{i j}^{(\alpha)} \frac{\partial u^{(\alpha)}}{\partial x_{j}} n_{i} V^{(\alpha)}\right] d s \tag{16}
\end{equation*}
$$

where $C$ is a constant such that $C=1$ if $(a, b) \in \Omega$ and if $(a, b) \in \partial \Omega$ then $0<C<1$. Also $V^{(\alpha)}$ satisfies the equation

$$
\begin{equation*}
\lambda_{i j}^{(\alpha)} \frac{\partial^{2} V^{(\alpha)}}{\partial x_{i} \partial x_{j}}+\rho^{(\alpha)} \omega^{2} V^{(\alpha)}=\delta\left(\mathbf{x}-\mathbf{x}_{0}\right) \tag{17}
\end{equation*}
$$

To obtain a solution to (16), it is helpful to proceed as follows. Let $z^{(\alpha)}=$ $x_{1}+\tau^{(\alpha)} x_{2}$ and $\bar{z}^{(\alpha)}=x_{1}+\bar{\tau}^{(\alpha)} x_{2}$ where $\tau^{(\alpha)}$ is the complex root with positive imaginary part of the quadratic

$$
\begin{equation*}
\lambda_{11}^{(\alpha)}+2 \lambda_{12}^{(\alpha)} \tau^{(\alpha)}+\lambda_{22}^{(\alpha)}\left(\tau^{(\alpha)}\right)^{2}=0 \tag{18}
\end{equation*}
$$

and the bar denotes the conjugate of a complex number. Then (17) transforms to

$$
\begin{equation*}
2\left[\lambda_{11}^{(\alpha)}+\lambda_{12}^{(\alpha)}\left(\tau^{(\alpha)}+\bar{\tau}^{(\alpha)}\right)+\tau^{(\alpha)} \bar{\tau}^{(\alpha)} \lambda_{22}^{(\alpha)}\right] \frac{\partial^{2} V^{(\alpha)}}{\partial z^{(\alpha)} \partial \bar{z}^{(\alpha)}}+\rho^{(\alpha)} \omega^{2} V^{(\alpha)}=\delta\left(\mathbf{x}-\mathbf{x}_{0}\right) \tag{19}
\end{equation*}
$$

Let $z^{(\alpha)}=\dot{x}_{1}^{(\alpha)}+\mathrm{i} \dot{x}_{2}^{(\alpha)}, \bar{z}^{(\alpha)}=\dot{x}_{1}^{(\alpha)}-\mathrm{i} \dot{x}_{2}^{(\alpha)}$ and $\tau^{(\alpha)}=\dot{\tau}^{(\alpha)}+\mathrm{i} \ddot{\tau}^{(\alpha)}$ where $\dot{x}_{1}^{(\alpha)}, \dot{x}_{2}^{(\alpha)}, \dot{\tau}^{(\alpha)}$ and $\ddot{\tau}^{(\alpha)}$ are real numbers. Then

$$
\begin{equation*}
\dot{x}_{1}^{(\alpha)}=x_{1}+\dot{\tau}^{(\alpha)} x_{2} \quad \text { and } \quad \dot{x}_{2}^{(\alpha)}=\ddot{\tau}^{(\alpha)} x_{2} \tag{20}
\end{equation*}
$$

Use of (20) in (19) provides

$$
\begin{equation*}
\frac{\partial^{2} V^{(\alpha)}}{\partial \dot{x}_{1}^{2}}+\frac{\partial^{2} V^{(\alpha)}}{\partial \dot{x}_{2}^{2}}+\bar{\omega}^{(\alpha)^{2}} V^{(\alpha)}=K^{(\alpha)} \delta\left(\mathbf{x}-\mathbf{x}_{0}\right) \tag{21}
\end{equation*}
$$

where

$$
\begin{equation*}
\bar{\omega}^{(\alpha)^{2}}=\frac{2 \omega^{2} \rho^{(\alpha)}}{\lambda_{11}^{(\alpha)}+\lambda_{12}^{(\alpha)}\left(\tau^{(\alpha)}+\bar{\tau}^{(\alpha)}\right)+\tau^{(\alpha)} \bar{\tau}^{(\alpha)} \lambda_{22}^{(\alpha)}} \tag{23}
\end{equation*}
$$

and

$$
\begin{equation*}
K^{(\alpha)}=\frac{2}{\lambda_{11}^{(\alpha)}+\lambda_{12}^{(\alpha)}\left(\tau^{(\alpha)}+\bar{\tau}^{(\alpha)}\right)+\tau^{(\alpha)} \bar{\tau}^{(\alpha)} \lambda_{22}^{(\alpha)}} \tag{24}
\end{equation*}
$$

A solution to (21) may be written in the form

$$
\begin{equation*}
V^{(\alpha)}=\frac{\mathrm{i}}{4} K^{(\alpha)} H_{0}^{(2)}\left(\bar{\omega}^{(\alpha)} R^{(\alpha)}\right) \tag{25}
\end{equation*}
$$

where $H_{0}^{(2)}$ denotes the Hankel function of the second kind and order zero and

$$
\begin{equation*}
R^{(\alpha)}=\sqrt{\left(\dot{x}_{1}^{(\alpha)}-\dot{a}^{(\alpha)}\right)^{2}+\left(\dot{x}_{2}^{(\alpha)}-\dot{b}^{(\alpha)}\right)^{2}} \tag{26}
\end{equation*}
$$

where $\dot{a}^{(\alpha)}=a+\dot{t}^{(\alpha)} b$ and $\dot{b}^{(\alpha)}=b \ddot{i}^{(\alpha)}$ so that

$$
\begin{equation*}
R^{(\alpha)}=\sqrt{\left(x_{1}+\dot{\tau}^{(\alpha)} x_{2}-a-\dot{\tau}^{(\alpha)} b\right)^{2}+\left(x_{2} \ddot{\tau}^{(\alpha)}-b \ddot{\tau}^{(\alpha)}\right)^{2}} . \tag{27}
\end{equation*}
$$

Hence the integral equation is given by (16) with the fundamental solution $V^{(\alpha)}$ given by (25).

Now any solution to (17) may be used in the integral equation (16). Here it is convenient to choose a solution to (17) which gives rise to the term $\lambda_{2 j}^{(\alpha)}\left(\partial V^{(\alpha)} / \partial x_{j}\right)$ equalling zero on $x_{2}=0$, it follows that the integral along the boundary $x_{2}=0$ in (16) will be zero. By image considerations it may be seen that a suitable choice of solution of (17) is

$$
\begin{equation*}
V^{(\alpha)}=\frac{\mathrm{i}}{4}\left[K^{(\alpha)} H_{0}^{(2)}\left(\bar{\omega}^{(\alpha)} R^{(\alpha)}\right)+K^{(\alpha)} H_{0}^{(2)}\left(\bar{\omega}^{(\alpha)} \bar{R}^{(\alpha)}\right)\right] \tag{28}
\end{equation*}
$$

where

$$
\begin{equation*}
R^{(\alpha)}=\sqrt{\left(x_{1}+\dot{\tau}^{(\alpha)} x_{2}-a-\dot{\tau}^{(\alpha)} b\right)^{2}+\left(x_{2} \dot{\tau}^{(\alpha)}-b \dot{\tau}^{(\alpha)}\right)^{2}} \tag{29}
\end{equation*}
$$

and

$$
\begin{equation*}
\bar{R}^{(\alpha)}=\sqrt{\left(x_{1}+\dot{\tau}^{(\alpha)} x_{2}-a-\dot{\tau}^{(\alpha)} b\right)^{2}+\left(x_{2} \dot{\tau}^{(\alpha)}+b \dot{\tau}^{(\alpha)}\right)^{2}} \tag{30}
\end{equation*}
$$

By applying (16) to region 1 with the Green's function (28) it is only necessary to integrate over the material interface between regions 1 and 2 . The integration along the $O x_{1}$ axis is zero so the only contribution is obtained from the interface. Similarly in region 2 it is necessary to integrate only on the interface between regions 1 and 2. In applying the integral equation (16), the boundary conditions over the interface involve the continuity of stress and displacement so that

$$
\begin{equation*}
u^{(1)}=u^{(2)}, \tag{31}
\end{equation*}
$$

and

$$
\begin{align*}
& \left(\lambda_{11}^{(1)} \frac{\partial u^{(1)}}{\partial x_{1}}+\lambda_{12}^{(1)} \frac{\partial u^{(1)}}{\partial x_{2}}\right) n_{1}+\left(\lambda_{21}^{(1)} \frac{\partial u^{(1)}}{\partial x_{1}}+\lambda_{22}^{(1)} \frac{\partial u^{(1)}}{\partial x_{2}}\right) n_{2} \\
& \quad=\left(\lambda_{11}^{(2)} \frac{\partial u^{(2)}}{\partial x_{1}}+\lambda_{12}^{(2)} \frac{\partial u^{(2)}}{\partial x_{2}}\right) n_{1}+\left(\lambda_{21}^{(2)} \frac{\partial u^{(2)}}{\partial x_{1}}+\lambda_{22}^{(2)} \frac{\partial u^{(2)}}{\partial x_{2}}\right) n_{2} \tag{32}
\end{align*}
$$

The conditions (31) and (32) can be used in conjunction with (16) to solve for the displacement and stress over the interface and the displacement along
the traction free surface $x_{2}=0$. Once this has been done, (16) gives the value of $u^{(\alpha)}(a, b)$ at all points $(a, b)$ in the half-space $x_{2}>0$.

## 4. Numerical results

Suppose region 2 (Figure 1) is defined by $x_{1}^{2}+x_{2}^{2} \leq a^{2}$ with $x_{2} \geq 0$, the material in region 1 has the elastic moduli $\lambda_{i j}^{(1)}$ and density $\rho^{(1)}$ and the material in region 2 has the elastic moduli $\lambda_{i j}^{(2)}$ and density $\rho^{(2)}$.

The material properties can be written as dimensionless quantities in the form

$$
\begin{equation*}
\dot{\lambda}_{i j}^{(\alpha)}=\lambda_{i j}^{(\alpha)} / \lambda_{11}^{(1)} \tag{33}
\end{equation*}
$$

and

$$
\begin{equation*}
\rho=\rho^{(2)} / \rho^{(1)} \tag{34}
\end{equation*}
$$

The ratio of the diameter $2 a$ of the alluvial valley to the wavelength $\Lambda$ of the incident wave is denoted by $\eta$ so that

$$
\begin{equation*}
\eta=2 a / \Lambda . \tag{35}
\end{equation*}
$$

Let $T$ be the duration of time for the initial wave to travel a distance of one wave length $\Lambda$ so that

$$
\begin{equation*}
T=\Lambda / \beta^{(1)} \tag{36}
\end{equation*}
$$

or from (1)

$$
\begin{equation*}
T=2 \pi / \omega . \tag{37}
\end{equation*}
$$

Use of (35), (36) and (37) provides

$$
\begin{equation*}
\omega=\eta \pi \beta^{(1)} / a \tag{38}
\end{equation*}
$$

All length measurements are made dimensionless by referring lengths to the valley's radius $a$. So $x_{1}^{\prime}=x_{1} / a$ and $x_{2}^{\prime}=x_{2} / a$. Using these dimensionless quantities, (16) becomes

$$
\begin{equation*}
C u^{(\alpha)}(c, d)=\int_{\partial \Omega}\left[\dot{\lambda}_{i j}^{(\alpha)} \frac{\partial \dot{V}^{(\alpha)}}{\partial x_{j}^{\prime}} n_{i} u^{(\alpha)}-\dot{\lambda}_{i j}^{(\alpha)} \frac{\partial u^{(\alpha)}}{\partial x_{j}^{\prime}} n_{i} \dot{V}^{(\alpha)}\right] d s \tag{39}
\end{equation*}
$$

where

$$
\begin{gather*}
\dot{V}^{(\alpha)}=\frac{i}{4}\left[\dot{K}^{(\alpha)} H_{0}^{(2)}\left(\dot{\omega}^{(\alpha)} \dot{R}^{(\alpha)}\right)+\dot{K}^{(\alpha)} H_{0}^{(2)}\left(\omega^{(\alpha)} \dot{R}^{(\alpha)}\right)\right],  \tag{40}\\
\dot{R}^{(\alpha)}=\sqrt{\left(x_{1}^{\prime}+\dot{\tau}^{(\alpha)} x_{2}^{\prime}-c^{\prime}-\dot{\tau}^{(\alpha)} d^{\prime}\right)^{2}+\left(x_{2}^{\prime} \ddot{i}^{(\alpha)}-d^{\prime} \dot{\tau}^{(\alpha)}\right)^{2}},  \tag{41}\\
\dot{R}^{(\alpha)}=\sqrt{\left(x_{1}^{\prime}+\dot{\tau}^{(\alpha)} x_{2}^{\prime}-c^{\prime}-\dot{\tau}^{(\alpha)} d^{\prime}\right)^{2}+\left(x_{2}^{\prime} \dot{\tau}^{(\alpha)}+d^{\prime} \dot{\tau}^{(\alpha)}\right)^{2}},  \tag{42}\\
\dot{K}^{(\alpha)}=\frac{2}{\left(\dot{\lambda}_{11}^{(\alpha)}+2 \dot{\lambda}_{12}^{(\alpha)} \dot{\tau}^{(\alpha)}+\left(\left[\dot{\tau}^{(\alpha)}\right]^{2}+\left[\dot{\tau}^{(\alpha)}\right]\right)^{2} \dot{\lambda}_{22}^{(\alpha)}\right)},  \tag{43}\\
{\left[\dot{\omega}^{(1)}\right]^{2}=\dot{K}^{(1)}\left[\pi \eta \dot{\beta}^{(1)}\right]^{2},}  \tag{44}\\
{\left[\dot{\beta}^{(1)}\right]^{2}=\dot{\lambda}_{11}^{(1)} \sin ^{2} \gamma_{I}+2 \dot{K}_{12}^{(1)} \rho\left[\pi \eta \dot{\beta}^{(1)}\right]^{2},}  \tag{45}\\
\dot{\tau}^{(\alpha)}=\dot{\lambda}_{12}^{(\alpha)} / \dot{\lambda}_{22}^{(\alpha)}, \tag{46}
\end{gather*}
$$

and

$$
\begin{equation*}
\left[\ddot{i}^{(\alpha)}\right]^{2}=\left(\frac{\dot{\dot{\lambda}}_{11}^{(\alpha)}}{\dot{\lambda}_{22}^{(\alpha)}}-\left(\frac{\dot{\lambda}_{12}^{(\alpha)}}{\dot{\lambda}_{22}^{(\alpha)}}\right)^{2}\right) . \tag{48}
\end{equation*}
$$

The displacement $u^{(\alpha)}(c, d)$ has amplitude $\chi$ which is the ratio of the displacement to the amplitude of the initial incident wave $u_{i}^{(\alpha)}$.

Since the initial incident wave has amplitude unity it follows that the displacement can be written as

$$
\begin{equation*}
u^{(\alpha)}=\chi \exp (\mathrm{i} \phi) \tag{49}
\end{equation*}
$$

In the numerical calculations, values of the displacement amplitude $\chi$ were obtained on the surface $x_{2}^{\prime}=0$, which is the place of interest in most considerations concerning earthquakes. The amplitude $\chi$ is a function of the parameters $\eta, \rho, \gamma_{I}$, and the moduli $\dot{\lambda}_{i j}^{(\alpha)}$.

If the material in either region 1 or region 2 is transversely isotropic and the $x_{1}^{*}$ and $x_{2}^{*}$ axes are the axes of symmetry for the material with the $x_{3}$ axis normal to the transverse plane, then using the transformation law for Cartesian tensors

$$
\begin{equation*}
\dot{\lambda}_{i j}^{(\alpha)}=a_{i m} a_{j n} \dot{\lambda}_{m n}^{*(\alpha)} \tag{50}
\end{equation*}
$$

where $\dot{\lambda}_{11}^{*(\alpha)}, \dot{\lambda}_{22}^{*(\alpha)}$ and $\dot{\lambda}_{12}^{*(\alpha)}=0$ are the elastic moduli referred to the $O x_{1}^{*} x_{2}^{*}$ frame and

$$
\left(a_{i j}\right)=\left(\begin{array}{cc}
\cos (\zeta) & \sin (\zeta)  \tag{51}\\
-\sin (\zeta) & \cos (\zeta)
\end{array}\right)
$$

where $\zeta$ is the angle between the $x_{1}^{*}$ axis and the $x_{1}$ axis (Figure 3).


Figure 3. Angle of anisotropy $\zeta$.
The numerical procedure used to solve the boundary integral equation (16) was a standard procedure used for equations of this type [4]. The interface boundary between regions 1 and 2 was divided into 80 equal segments and the unknowns in (16) assumed to be constant over each segment. Equation (16) was thus reduced to a system of linear algebraic equations for the values of the unknown variables on each segment. Once this system was solved, (16) was used to determine the displacement on the line $x_{2}=0$. Results for the case when both regions 1 and 2 are isotropic compared favourably with the results published in [6]. Further results for the isotropic case compared well with values calculated by the method published in [7].

The numerical solutions displayed in Figures 4, 5, 6, 7 and 8 are for the case when region 1 is an isotropic material with $\dot{\lambda}_{11}^{(1)}=1, \dot{\lambda}_{22}^{(1)}=1, \dot{\lambda}_{12}^{(1)}=0$ and $\rho=2 / 3$. For Figures 4, 5 and 6, region 2 has the parameters $\dot{\lambda}_{11}^{*(2)}=1 / 6$,


Figure 4. Effect of anisotropy, $\gamma_{I}=60^{\circ}$.


Figure 5. Effect of anisotropy, $\gamma_{I}=30^{\circ}$.


Figure 6. Effect of anisotropy, $\gamma_{I}=0^{\circ}$.
$\dot{\lambda}_{22}^{*(2)}=1 / 3, \dot{\lambda}_{12}^{*(2)}=0$ and $\eta=1.0$. Calculations were carried out for the angles $\zeta=0^{\circ}, 15^{\circ}, 30^{\circ}, 45^{\circ}, 60^{\circ}, 75^{\circ}$ and $90^{\circ}$. In Figure $4, \gamma_{I}=60^{\circ}$, while in Figure 5, $\gamma_{I}=30^{\circ}$ and in Figure 6, $\gamma_{I}=0^{\circ}$. These figures illustrate the effect of anisotropy in the alluvial valley on the surface displacements.

The results show how anisotropy effects the location of the largest displacement amplitude. For example, it is clear from Figure 6 that when $\zeta=0^{\circ}$ and $90^{\circ}$, the displacement is symmetric about $x_{1}^{\prime}=0$ but when $0<\zeta<90$, the maximum displacement lies in the region $-2<x_{1}^{\prime}<0$.

Results shown in Figure 7 are for the same parameters as Figure 4 with $\gamma_{I}=60^{\circ}$, but with the semi-circular geometry in region 2 replaced by a


Figure 7. Effect of a rectangular valley.


Figure 8. Effect of wavelength.
rectangle with length $2 a$ along the $x_{1}$ axis and depth $a$ on the $x_{2}$ axis.
Figure 8 shows results obtained using the same parameters as in Figure 4 but with $\zeta=75^{\circ}$ and $\eta=0.1,0.5,1.0,1.5,2.0$. The results show that in this case, the number of peaks and troughs in the displacement amplitude varied significantly for different wavelenghts.

In Figure 9, anisotropy was introduced to the surrounding material in region 1. The alluvial valley had the same parameters as for Figure 7 but the outside material had the elastic moduli $\dot{\lambda}_{11}^{*(1)}=1 / 6, \dot{\lambda}_{22}^{*(1)}=1 / 3$ and $\eta=1.0$. The angle $\zeta$ was varied in steps of $15^{\circ}$ and the results showed that


Figure 9. Effect of anisotropy in region 1.
the angle $\zeta$ for region 1 had little effect on the displacement amplitude on the surface in region 2.

## 5. Conclusion

A boundary element method for the scattering and diffraction of SH waves by anisotropic alluvial valleys with an arbitrary cross section has been constructed in this paper by the use of the fundamental solution given in (28). The use of (16) allows the surface displacement to be calculated. The method includes previously obtained methods as special cases. For these cases the numerical results compared well with those given by Trifunac's method and by Sanchez-Sesma and Esquivel [6].

For the materials considered in numerical examples the results showed that anisotropy in the valley has a significant effect on the amplitude of the surface displacement.

## References

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